



Report of Subsurface Exploration and Infiltration Evaluation

Hightstown Redevelopment

Block 21, Lots 1 -14 and 26,
And Block 30, Lots 1-7 and 10
Bank Street and North Main Street
Township of Hightstown, Mercer County, New Jersey

July 18, 2017

Prepared For

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Prepared By

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A handwritten signature in black ink that reads 'Michael Carnivale III'.

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TABLE OF CONTENTS

1.0 INTRODUCTION..... 1
2.0 SITE DESCRIPTION..... 1
3.0 SCOPE OF SERVICES..... 2
4.0 SUBSURFACE EXPLORATION 3
5.0 SUBSURFACE CONDITIONS 4
6.0 SOIL INFILTRATION EVALUATION 4
7.0 GENERAL CONSTRUCTION DISCUSSION..... 5
8.0 CLOSING 6
9.0 LIMITATIONS 6

FIGURE

Exploration Location Plan Figure No. 1

APPENDICES

APPENDIX A Test Pit Logs
APPENDIX B Laboratory Testing



1.0 INTRODUCTION

This report presents the results of the geotechnical evaluation performed in June 2017, at the project site with respect to proposed stormwater management areas and infiltration rates for use in design. Maser Consulting P.A. understands that the purpose of the proposed project is to construct a mixed use development consisting of multi-family units and retail space along with typical appurtenant site improvements on an existing 6.9 acres of land located between North Academy Street, Bank Street and North Main Street in Hightstown Township, Mercer County, New Jersey.

Infiltration rate recommendations provided in this report are based on review of published data, accepted engineering practice, and field observations. Maser Consulting P.A. has evaluated the subsurface conditions at the site, and provides an evaluation of potential infiltration rates for soils encountered at depth within the area of the proposed stormwater management systems and design seasonal high water levels.

2.0 SITE DESCRIPTION

The subject project site is an approximately 6.9 acres site bounded by North Academy Street, Bank Street and North Main Street, in Hightstown Township, Mercer County, New Jersey and is referred to as Block 21, Lots 1-14 and 26, and Block 30, Lots 1-7 and 10 on the Hightstown Township Tax Maps. Rocky Brook runs through the center of the property. The current use consists primarily of warehouse structures (RTL Merchandising, Moving, Storage and Decorations).

We understand the purpose of the proposed project is to redevelop the site with a combination of multi-family units and retail space along with typical appurtenant site improvements.



3.0 SCOPE OF SERVICES

In order to evaluate the subsurface conditions within the influence of the proposed stormwater management areas and to subsequently provide consultation regarding anticipated subsurface infiltration rates and design estimated seasonal high water levels (ESHWL), we performed the following scope of services:

- a) Engaged the services of an excavation contractor to excavate test pits for exploration of subsurface soil and groundwater conditions within the proposed stormwater management areas;
- b) Provided full-time technical observation of the excavation work;
- c) Obtained representative soil samples encountered within the zone of influence of the proposed construction;
- d) Evaluated the field data and prepared test pit logs showing the types of soils observed, depths to groundwater, and depths to estimated seasonal high groundwater;
- e) Performed a combination of laboratory testing on select soil samples to evaluate groundwater infiltration rates for the subgrade soils; and
- f) Provided a Subsurface Exploration and Infiltration Evaluation Report that reviews potential soil infiltration rates for design and groundwater considerations for the proposed basin requirements.



4.0 SUBSURFACE EXPLORATION

The subsurface conditions were evaluated on June 16, 2017, through the excavation of a total of four (4) test pits, labeled TP-1 through TP-4, that were advanced to termination depths of approximately 15 feet (180 inches) below ground surface by Viersma & Sons of Allamuchy, NJ, using a CAT 315F trackhoe. Test pits for the exploration were excavated at the locations shown on the Exploration Location Plan, Figure No. 1.

Representatives from Maser Consulting's Geotechnical Department observed the test pit excavations. Soils encountered were classified in the field in accordance with N.J.A.C. 7:9A, Subchapter 5.3, Terminology Required for Soil Logs. The Burmister Soil Classification System was also used. Representative soil samples of strata encountered were collected and returned to Maser Consulting's Red Bank laboratory facilities for further evaluation and analyses. Details pertaining to the subsurface conditions encountered are presented on the Test Pit Logs in Appendix A.

The depth of groundwater was measured from the ground surface to the point of observed seepage or consistent soil moisture, and was encountered in the test pits at depths ranging from approximately 3.75 to 10 feet below existing grade. The subsurface strata were also evaluated with respect to mottling and soil staining, to determine if seasonal high groundwater levels extended into the test pit depths. Staining and mottling within a soil stratum can indicate seasonal high water level (SHWL) fluctuations, but is also found along wormholes, as a result of prior farming practices, or as an indication of geologic depositional factors. Evidence of SHWL was encountered at depths ranging from approximately 12 inches to 30 inches below existing ground surface within the four (4) test pits excavated as part of this exploration.

Please refer to Table 1 for a summary of depths to the groundwater table and to the estimated seasonal high water level (ESHWL). Soil moisture and groundwater conditions should be expected to fluctuate with season, precipitation amounts, and other on-site and off-site factors including site utilization.



**HIGHTSTOWN REDEVELOPMENT
MC PROJECT NO. 16001094A**

TABLE 1 DEPTH TO GWT AND ESHWL SUMMARY			
Test Pit ID	Approx. Ground Surface Elev. (ft)	Depth to Groundwater Water Table, GWT (in)	Depth to Estimated Seasonal High Water Level, ESHWL (in)
TP-1	±84.0	96	12
TP-2	±88.0	102	20
TP-3	±86.0	120	16
TP-4	±85.5	45	30

5.0 SUBSURFACE CONDITIONS

The soils encountered during our exploration consisted of the existing pavement layer, approximately 6 to 12 inches thick, consisting of light brownish gray loamy sand with up to 50% gravel, underlain by layers of grayish brown, brown and yellowish brown sandy loam. Loam and silty clay loam, with varying amounts of gravel (up to 25%). These layers were underlain by a black clay loam, which was encountered at depths ranging from approximately 7.5 to 12 feet below existing grade and extended to the termination depth of the test pits at approximately 15 feet below existing grade.

6.0 SOIL INFILTRATION EVALUATION

Selected soil samples were tested by the Maser Consulting Geotechnical Laboratory in Red Bank, New Jersey. The testing consisted of eight (8) Tube Permeameter Tests performed to estimate the infiltration rate of groundwater through the soils at depth. Tube Permeameter testing was performed in accordance with New Jersey Administrative Code (N.J.A.C.) 7:9A-6.2 and BMP-E requirements. The soil samples were selected based on review of test pit logs by design personnel, the proposed infiltration depths, and comparison to other strata encountered at each test pit location. The tube samples were collected from the soils directly by inserting the sample tube into the ground and retrieving the tube by excavating the soils surrounding it.

Infiltration test results are summarized in Table 2 on the following page, with laboratory testing results provided in Appendix B.



**HIGHTSTOWN REDEVELOPMENT
MC PROJECT NO. 16001094A**

TABLE 2 INFILTRATION TEST SUMMARY			
Test Pit ID	Approx. Ground Surface Elev. (ft)	Depth below Existing Grade (in)	Infiltration Rate (in/hr)
TP-1	±84.0	72	0.0
		96	0.0
TP-2	±88.0	43	11.4 / 18.2
		96	0.0
		144	0.0
TP-3	±86.0	68	0.0
TP-4	±85.5	39	5.6 / 3.7
		96	0.0

7.0 GENERAL CONSTRUCTION DISCUSSION

Although representative samples of near surface granular soils indicated good infiltration rates, potential indicators of the estimated seasonal high water level (ESHWL) were observed at very shallow depths (12 to 30 inches below ground surface) in the test pits excavated as part of this subsurface exploration. Given that the ESHWL was found to be within 24 inches of the existing ground surface in three of the four test pits, it is our opinion that the site is not viable for infiltration purposes per BMP-E requirements.

Excavated soils with high silt and clay contents are unsuitable for use as structural fill throughout the site. Soils containing significant quantities of organic materials may need to be removed from the site and disposed in a manner consistent with local, state and federal regulations. Stripped topsoil and cohesive materials may be used to raise site grades in lawn areas, but may be difficult to re-handle and place in a manner that will minimize post-construction subsidence. During periods of inclement weather, placing and compaction difficulties will also occur since the materials, in general, will be moisture sensitive. Granular materials encountered during site earthwork operations should be segregated for reuse as general fills for this project.



8.0 CLOSING

Successful construction of the project will require competent field observation of the construction operations. Earthwork, including clearing and grubbing, subgrade identification, grading, and fill placement should be observed by a competent individual familiar with the recommendations contained herein. We are available to perform construction observation services, if requested.

The recommendations contained herein are contingent upon the actual field conditions being consistent with those encountered during our field exploration. Should any variation in the anticipated conditions be encountered or site regrading be proposed, Maser Consulting P.A. should be notified immediately to determine what impact the changed conditions may have upon the presented recommendations.

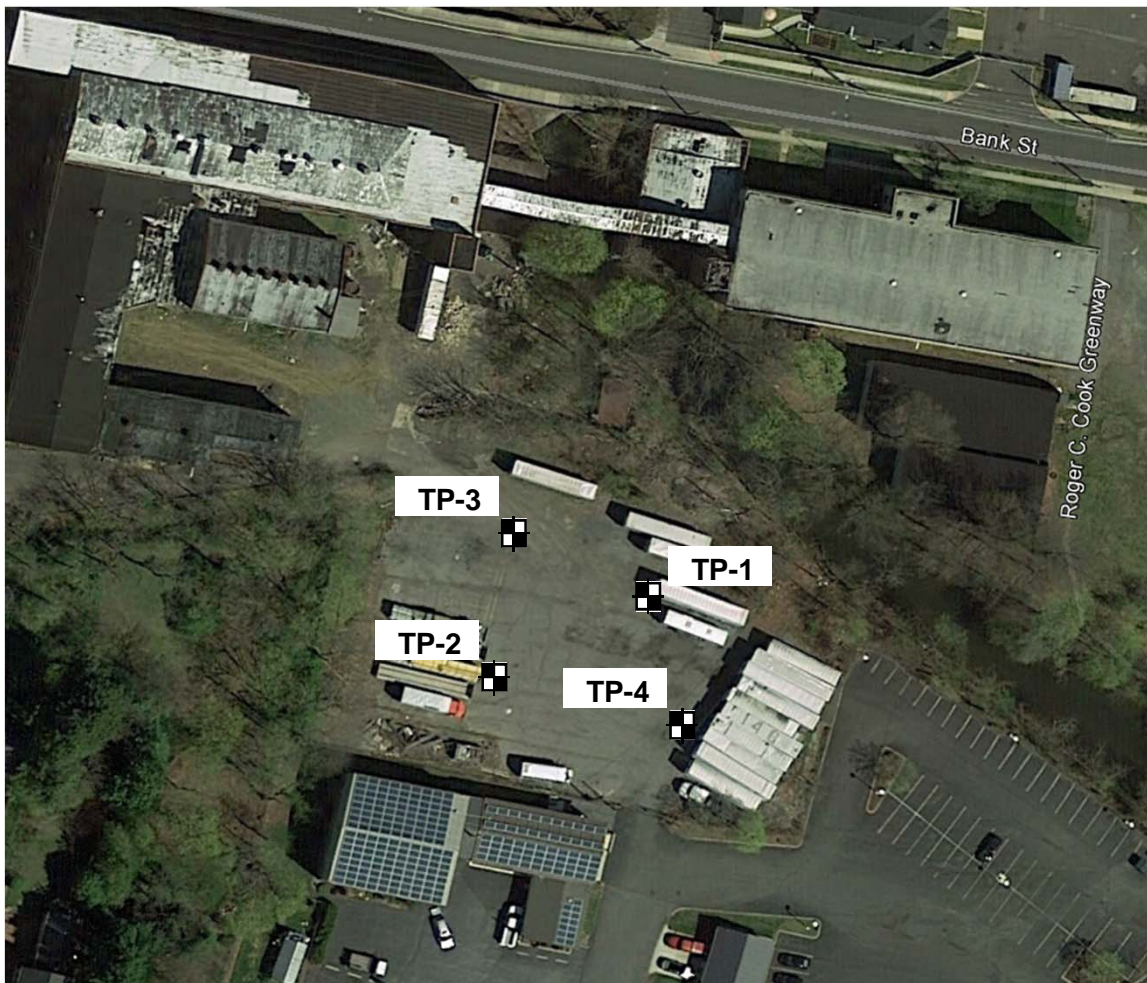
9.0 LIMITATIONS

Services performed by Maser Consulting P.A. during this project have been conducted in a manner consistent with the level of care and skill ordinarily exercised by members of the profession currently practicing in the same locality under similar conditions. No other representation, expressed or implied, and no warranty or guarantee is included or intended in the services provided. This is not an Environmental Assessment.

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PLAN
NORTH



LEGEND:

 **TP-1** TEST PIT LOCATION (APPROX.)

NOTES:

- 1.) *BASE PLAN OBTAINED FROM GOOGLE EARTH PRO IMAGE, BY MASER CONSULTING, PA, REVISED JUNE 20, 2017.
- 2.) THIS DRAWING IS PART OF MASER's GEOTECHNICAL REPORT (PROJECT NO. 16001094A) DATED JULY 2017 AND SHOULD ONLY BE USED IN CONJUNCTION WITH THE REPORT.



Consulting, Municipal & Environmental Engineers
Planners ■ Surveyors ■ Landscape Architects

New Jersey New York Pennsylvania Virginia
Customer Loyalty through Client Satisfaction

Title:

EXPLORATION LOCATION PLAN

Project:

HIGHTSTOWN REDEVELOPMENT
101 Bank Street
Borough of Hightstown
Mercer County, NJ

Drawn By:

PA

Checked By:

MC

Project No.:

16001094A

Scale:

N.T.S.

Date:

06-20-2017

Figure No.:

1



APPENDIX A

TEST PIT LOGS

VISUAL IDENTIFICATION OF SAMPLES
(Burmister Soil Classification System)

I. Definition of Soil Components and Fractions

<u>Material</u>	<u>Symbol</u>	<u>Fraction</u>	<u>Sieve Size</u>	<u>Definition</u>
Boulders	Bldr	-----	9" +	Material retained on 9" sieve.
Cobbles	Cbl	----	3" to 9"	Material passing the 9" sieve and retained on the 3" sieve.
Gravel	G	coarse (c) medium (m) fine (f)	1" to 3" 3/8" to 1" No. 10 to 3/8"	Material passing the 3" sieve and retained on the No. 10 sieve.
Sand	S	coarse (c) medium (m) fine (f)	No. 30 to No. 10 No. 60 to No. 30 No. 200 to No. 60	Material passing the No. 10 sieve and retained on the No. 200 sieve.
Silt	\$	---	Passing No. 200 (0.075 mm)	Material passing the No. 200 sieve that is non-plastic in character and exhibits little or no strength when air dried.
Clayey SILT	Cy\$	Slight (SL)	1 to 5	Clay - Soil
SILT & CLAY	\$ & C	Low (L)	5 to 10	Material passing the No. 200 which can be made to exhibit plasticity and clay qualities within a certain range of moisture content, and which exhibits considerable strength when air-dried.
CLAY & SILT	C & \$	Medium (M)	10 to 20	
Silty CLAY	\$yC	High (H)	20 to 40	
CLAY	C	Very High (VH)	40 Plus	
Organic Silt	(O\$)			Material passing the No. 200 sieve which exhibits plastic properties within a certain range of moisture content, and exhibits fine granular and organic characteristics.

II. Definition of Component Proportions

<u>Component</u>	<u>Written</u>	<u>Proportions</u>	<u>Symbol</u>	<u>Percentage Range by Weight*</u>
Principal	CAPITALS	---		50 or more
Minor	Lower Case	and	a.	35 to 50
		some	s.	20 to 35
		little	l.	10 to 20
		trace	t.	1 to 10

* Minus sign (-) lower limit, plus sign (+) upper limit, no sign middle range.



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TEST PIT No. TP-1

DATE EXCAVATED: 6/15/17
SURFACE ELEVATION: 84.0+/-

Project: Hightstown Redevelopment
Location: Hightstown, NJ.
Job Number: 16001094A

EXCAVATED BY: Kevin Viersma
EQUIPMENT USED: CAT 315F Trackhoe
INSPECTED BY: Pavle A./Anastasia D.

DEPTH (ft)	DEPTH (in)	DESCRIPTION	REMARKS
0	3	(10 YR 6/2) Lt. Brownish Gray Loamy Sand. >50% Gravel. (Existing Pavement) (Dry).	
	6		
	9	(10 YR 3/2) Very Dk. Grayish Brown Sandy Loam. Subangular-Blocky, Friable. 20% Gravel. Frequent Fine Roots. (Possible Topsoil, Moist).	
	12		
	24	(7.5 YR 5/6) Strong Brown Loam. Subangular-Blocky, Friable. 10% Gravel. Common, Distinct, Med-Fine (5 YR 5/8) Yellowish Red Mottles at 12"-45". (Moist).	
	36		
	48		
5	60	(10 YR 4/2) Dk. Grayish Brown Silty Clay Loam. Subangular-Blocky, Friable. Common, Distinct, Fine (5 YR 5/8) Yellowish Red Mottles at 45"-90". (Moist).	
	72		
	84		
	96	(7.5 YR 2.5/1) Black Clay Loam. Massive, Firm. (Wet throughout).	
	108		
10	120		
	132		
	144		
	156		
	168	END OF TEST PIT AT 180 INCHES	
15	180		
	192	N.E. = Not Encountered N.A. = Not Applicable	
	204		
	216		
	228		
	240		
20	240		

GROUNDWATER:	DEPTH (ft.)	DATE
First Encountered	<u>8.0</u>	<u>6/15/17</u>
At Completion (0 hrs.)	<u>8.0</u>	<u>6/15/17</u>
After Completion (>24 hrs.)	<u>N.A.</u>	<u>6/15/17</u>

ESTIMATED DEPTH TO SEASONAL HIGH GROUNDWATER: 12 Inches

TEST PIT No. TP-1



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TEST PIT No. TP-2

DATE EXCAVATED: 6/15/17
SURFACE ELEVATION: 88.0+/-

Project: Hightstown Redevelopment
Location: Hightstown, NJ.
Job Number: 16001094A

EXCAVATED BY: Kevin Viersma
EQUIPMENT USED: CAT 315F Trackhoe
INSPECTED BY: Pavle A./Anastasia D.

DEPTH (ft)	DEPTH (in)	DESCRIPTION	REMARKS
0	0	(10 YR 6/2) Lt. Brownish Gray Loamy Sand. >50% Gravel.	
	3	(Existing Pavement) (Dry).	
	6		
	9		
	12		
	24	(7.5 YR 3/1) Very Dk. Gray Sandy Loam. Subangular-Blocky, Friable. 20% Gravel. Frequent Fine Roots.	
	36	Common, Distinct, Med-Fine (10 YR 4/6) Dk. Yellowish Brown Mottles at 20"-46". (Possible Fill) (Possibly Organic) (Moist).	
	46		
5	60		
	72		
	84		
	96	(7.5 YR 4/4) Brown Silty Clay Loam. Subangular-Blocky, Friable. 10% Gravel. Common, Distinct, Fine (5 YR 5/8) Yellowish Red Mottles at 46"-102".	
	108	Wet throughout at 102"-144".	
10	120		
	132		
	144		
	156	(7.5 YR 2.5/1) Black Clay Loam. Massive, Firm.	
	168	(Wet throughout).	
15	180		
	192		
	204		
	216		
	228		
20	240		

END OF TEST PIT
AT 180 INCHES

N.E. = Not Encountered
N.A. = Not Applicable

GROUNDWATER:	DEPTH (ft.)	DATE
First Encountered	<u>8.5</u>	<u>6/15/17</u>
At Completion (0 hrs.)	<u>8.5</u>	<u>6/15/17</u>
After Completion (>24 hrs.)	<u>N.A.</u>	<u>6/15/17</u>

ESTIMATED DEPTH TO SEASONAL HIGH GROUNDWATER: 20 Inches

TEST PIT No. TP-2



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TEST PIT No. TP-3

DATE EXCAVATED: 6/15/17
SURFACE ELEVATION: 86.0+/-

Project: Hightstown Redevelopment
Location: Hightstown, NJ.
Job Number: 16001094A

EXCAVATED BY: Kevin Viersma
EQUIPMENT USED: CAT 315F Trackhoe
INSPECTED BY: Pavle A./Anastasia D.

DEPTH (ft)	DEPTH (in)	DESCRIPTION	REMARKS
0	3	(10 YR 6/2) Lt. Brownish Gray Loamy Sand. >50% Gravel.	
	6	(Existing Pavement) (Dry).	
	9		
	12		
	22"	(10 YR 3/3) Dk. Brown Sandy Loam. Subangular-Blocky, Friable. 25% Gravel. Common, Distinct, Med-Fine (5 YR 5/8) Yellowish Red Mottles at 16"-22".	
	35"	(Possible Fill) (Moist).	
	48	(10 YR 3/2) Very Dk. Grayish Brown Sandy Loam. Subangular-Blocky, Firm. 20% Gravel. Frequent Medium-Fine Roots. Common, Distinct, Med-Fine (7.5 YR 6/8) Reddish Yellow Mottles at 22"-35".	
5	60	(Possible Fill) (Moist).	
	72		
	84	(10 YR 2/2) Very Dk. Brown Sandy Loam. Subangular-Blocky, Friable. 20% Gravel. Few Fine Roots. Few, Distinct, Med-Fine (7.5 YR 6/8) Reddish Yellow Mottles at 35"-48". (Moist).	
	96		
	108	(10 YR 4/4) Dk. Yellowish Brown Silty Clay Loam. Subangular-Blocky, Firm. 10% Gravel. Common, Distinct, Med-Fine Mottles at 53"-120". (Moist).	
10	120		
	132		
	144		
	156	(7.5 YR 2.5/1) Black Clay Loam. Massive, Firm. Lt. Seepage at 127"-144". (Wet throughout).	
	168		
15	180		
	192	END OF TEST PIT AT 180 INCHES	
	204		
	216	* Seepage due to perched condition.	
	228	N.E. = Not Encountered N.A. = Not Applicable	
20	240		

GROUNDWATER:	DEPTH (ft.)	DATE
First Encountered	<u>▽ 10.0</u>	<u>6/15/17</u>
At Completion (0 hrs.)	<u>▽ 10.0</u>	<u>6/15/17</u>
After Completion (>24 hrs.)	<u>▽ N.A.</u>	<u>6/15/17</u>

ESTIMATED DEPTH TO SEASONAL HIGH GROUNDWATER: 16 inches

TEST PIT No. TP-3



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TEST PIT No. TP-4

DATE EXCAVATED: 6/15/17

SURFACE ELEVATION: 85.5+/-

Project: Hightstown Redevelopment

Location: Hightstown, NJ.

Job Number: 16001094A

EXCAVATED BY: Kevin Viersma

EQUIPMENT USED: CAT 315F Trackhoe

INSPECTED BY: Pavle A./Anastasia D.

DEPTH (ft)	DEPTH (in)	DESCRIPTION	REMARKS
0	0	(10 YR 6/2) Lt. Brownish Gray Loamy Sand. >50% Gravel. (Existing Pavement) (Dry).	
	3		
	6		
	9		
	12		
	15		
	18		
	21		
	24	(10 YR 5/8) Brown Sandy Loam. Subangular-Blocky, Loose. 25% Gravel. (Moist throughout).	
	27		
	30		
	33		
	36		
	39		
	42		
	45		
5	48		
	51		
	54		
	57		
	60	(10 YR 4/6) Dk. Yellowish Brown Loam. Subangular-Blocky, Firm. Common, Distinct, Fine (5 YR 4/4) Reddish Brown Mottles at 30"-45". Lt. Seepage at 45"-80". Wet throughout at 80"-98".	
	63		
	66		
	69		
	72		
	75		
	78		
	81		
	84		
	87		
	90		
	93		
	96	98"	
	99		
	102		
	105		
	108		
10	111		
	114		
	117		
	120		
	123		
	126		
	129		
	132		
	135		
	138		
	141		
	144		
	147		
	150		
	153		
	156	(7.5 YR 2.5/1) Black Clay Loam. Massive, Firm. (Wet throughout).	
	159		
	162		
	165		
	168		
15	171		
	174		
	177		
	180		
	183		
	186		
	189		
	192		
	195		
	198		
	201		
	204		
	207		
	210		
	213		
	216		
	219		
	222		
	225		
	228		
	231		
	234		
	237		
20	240		

END OF TEST PIT
AT 180 INCHES

* Seepage due to perched condition.

N.E. = Not Encountered

N.A. = Not Applicable

GROUNDWATER: DEPTH (ft.) DATE

First Encountered ▽ 3.75 6/15/17

At Completion (0 hrs.) ▽ 3.75 6/15/17

After Completion (>24 hrs.) ▽ N.A. 6/15/17

ESTIMATED DEPTH TO SEASONAL HIGH GROUNDWATER: 30 Inches

TEST PIT No. TP-4



APPENDIX B

LABORATORY TESTING

TUBE PERMEAMETER TEST DATA

Project Name:	Hightstown Redevelopment	Project Number:	16001094A
Block:	21	Municipality:	Hightstown
Lot:	21	County:	Mercer
Test Number:	S-1	Date Collected:	6/15/2017
Material Tested:	TP-1	Replicate:	A
Depth of Sample:	72"	Sample Type:	Undisturbed

1. **Sample Dimensions** Radius = 2.54 cm
 Length = 4.25 in

2. **Measurements** Tube Weight = 358.68 g
 Total Weight = 722.91 g
 tube #: A20 Soil Weight = 364.23 g

Volume = Length * 2.54 cm/inch * π * Radius²
 Volume = 218.69

Bulk Density = Soil Weight / Volume
 Bulk Density = 1.67

Height of Water Level above Rim of Test Basin (inches)

At beginning of interval: 4.25 H₁
 At end of interval: 4.25 H₂

3. **Test Data**

	<u>Time Begin,</u> T ₁	<u>Time End,</u> T ₂	<u>Test Length</u> (min)	<u>Δ Height</u> (in)
a.	0:00	2:00:00	120	0.000
b.	0:00	2:00:00	120	0.000
c.	0:00	2:00:00	120	0.000
d.	0:00	2:00:00	120	0.000
		av =	120	0

4. **Permeability Calculation** K (in/hr) = 60 min/hr * r²/R² * L (in)/T (min) * ln (H₁/H₂)
 K = 0.00 in/hr = Soil Permeability Class K0

5. Any **Defects** in Sample: No

6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water pollution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.

Signature of Professional Engineer _____

License #

TUBE PERMEAMETER TEST DATA

Project Name:	Hightstown Redevelopment	Project Number:	16001094A
Block:	21	Municipality:	Hightstown
Lot:	21	County:	Mercer
Test Number:	S-1	Date Collected:	6/15/2017
Material Tested:	TP-1	Replicate:	B
Depth of Sample:	72"	Sample Type:	Undisturbed

1. **Sample Dimensions** Radius = 2.54 cm
 Length = 4.000 in

2. **Measurements** Tube Weight = 358.43 g
 Total Weight = 696.45 g
 tube #: Bm-69 Soil Weight = 338.02 g

Volume = Length * 2.54 cm/inch * π * Radius²
 Volume = 205.82

Bulk Density = Soil Weight / Volume
 Bulk Density = 1.64

Height of Water Level above Rim of Test Basin (inches)

At beginning of interval: 4.125 H₁
 At end of interval: 4.125 H₂

3. **Test Data**

	<u>Time Begin,</u> T ₁	<u>Time End,</u> T ₂	<u>Test Length</u> (min)	<u>Δ Height</u> (in)
a.	0:00	02:00:00	120	0.000
b.	0:00	02:00:00	120	0.000
c.	0:00	02:00:00	120	0.000
d.	0:00	02:00:00	120	0.000
		av =	120	0

4. **Permeability Calculation** K (in/hr) = 60 min/hr * r²/R² * L (in)/T (min) * ln (H₁/H₂)
 K = 0.00 in/hr = Soil Permeability Class K0

5. Any **Defects** in Sample: No

6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water pollution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.

Signature of Professional Engineer _____

License #

TUBE PERMEAMETER TEST DATA

Project Name:	Hightstown Redevelopment	Project Number:	16001094A
Block:	21	Municipality:	Hightstown
Lot:	21	County:	Mercer
Test Number:	S-2	Date Collected:	6/15/2017
Material Tested:	TP-1	Replicate:	B
Depth of Sample:	96"	Sample Type:	Undisturbed

1. **Sample Dimensions** Radius = 2.54 cm
 Length = 4.000 in

2. **Measurements** Tube Weight = 362.12 g
 Total Weight = 746.86 g
 tube #: PA-1 Soil Weight = 384.74 g

Volume = Length * 2.54 cm/inch * π * Radius²
 Volume = 205.82

Bulk Density = Soil Weight / Volume
 Bulk Density = 1.87

Height of Water Level above Rim of Test Basin (inches)

At beginning of interval: 4.25 H₁
 At end of interval: 4.25 H₂

3. **Test Data**

	<u>Time Begin,</u> T ₁	<u>Time End,</u> T ₂	<u>Test Length</u> (min)	<u>Δ Height</u> (in)
a.	0:00	02:00:00	120	0.000
b.	0:00	02:00:00	120	0.000
c.	0:00	02:00:00	120	0.000
d.	0:00	02:00:00	120	0.000
		av =	120	0

4. **Permeability Calculation** K (in/hr) = 60 min/hr * r²/R² * L (in)/T (min) * ln (H₁/H₂)
 K = 0.00 in/hr = Soil Permeability Class K0

5. Any **Defects** in Sample: No

6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water pollution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.

Signature of Professional Engineer _____

License #

TUBE PERMEAMETER TEST DATA

Project Name:	Hightstown Redevelopment	Project Number:	16001094A
Block:	21	Municipality:	Hightstown
Lot:	21	County:	Mercer
Test Number:	S-1	Date Collected:	6/15/2017
Material Tested:	TP-2	Replicate:	B
Depth of Sample:	43"	Sample Type:	Undisturbed

1. **Sample Dimensions** Radius = 2.54 cm
 Length = 4.500 in

2. **Measurements** Tube Weight = 355.35 g
 Total Weight = 747.2 g
 tube #: Bm-6 Soil Weight = 391.85 g

Volume = Length * 2.54 cm/inch * π * Radius²
 Volume = 231.55

Bulk Density = Soil Weight / Volume
 Bulk Density = 1.69

Height of Water Level above Rim of Test Basin (inches)

At beginning of interval: 3.875 H₁
 At end of interval: 2.625 H₂

3. **Test Data**

	<u>Time Begin,</u> T ₁	<u>Time End,</u> T ₂	<u>Test Length</u> (min)	<u>Δ Height</u> (in)
a.	0:00	00:05:15	5.25	1.250
b.	0:00	00:06:03	6.05	1.250
c.	0:00	00:05:50	5.83	1.250
d.	0:00	00:05:42	5.7	1.250
		av =	5.7075	1.25

4. **Permeability Calculation** K (in/hr) = 60 min/hr * r²/R² * L (in)/T (min) * ln (H₁/H₂)
 K = 18.42 in/hr = Soil Permeability Class K4

5. Any **Defects** in Sample: No

6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water pollution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.

Signature of Professional Engineer _____ License #

TUBE PERMEAMETER TEST DATA

Project Name:	Hightstown Redevelopment	Project Number:	16001094A
Block:	21	Municipality:	Hightstown
Lot:	21	County:	Mercer
Test Number:	S-2	Date Collected:	6/15/2017
Material Tested:	TP-2	Replicate:	B
Depth of Sample:	96"	Sample Type:	Undisturbed

1. **Sample Dimensions** Radius = 2.54 cm
 Length = 4.500 in

2. **Measurements** Tube Weight = 354.77 g
 Total Weight = 772.35 g
 tube #: Bm-41 Soil Weight = 417.58 g

Volume = Length * 2.54 cm/inch * π * Radius²
 Volume = 231.55

Bulk Density = Soil Weight / Volume
 Bulk Density = 1.80

Height of Water Level above Rim of Test Basin (inches)

At beginning of interval: 4 H₁
 At end of interval: 4 H₂

3. **Test Data**

	<u>Time Begin,</u> T ₁	<u>Time End,</u> T ₂	<u>Test Length</u> (min)	<u>Δ Height</u> (in)
a.	0:00	02:00:00	120	0.000
b.	0:00	02:00:00	120	0.000
c.	0:00	02:00:00	120	0.000
d.	0:00	02:00:00	120	0.000
		av =	120	0

4. **Permeability Calculation** K (in/hr) = 60 min/hr * r²/R² * L (in)/T (min) * ln (H₁/H₂)
 K = 0.00 in/hr = Soil Permeability Class K0

5. Any **Defects** in Sample: No

6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water pollution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.

Signature of Professional Engineer _____

License #

TUBE PERMEAMETER TEST DATA

Project Name:	Hightstown Redevelopment	Project Number:	16001094A
Block:	21	Municipality:	Hightstown
Lot:	21	County:	Mercer
Test Number:	S-3	Date Collected:	6/15/2017
Material Tested:	TP-2	Replicate:	B
Depth of Sample:	144"	Sample Type:	Undisturbed

1. **Sample Dimensions** Radius = 2.54 cm
 Length = 4.500 in

2. **Measurements** Tube Weight = 355.14 g
 Total Weight = 767.59 g
 tube #: AV-1 Soil Weight = 412.45 g

Volume = Length * 2.54 cm/inch * π * Radius²
 Volume = 231.55

Bulk Density = Soil Weight / Volume
 Bulk Density = 1.78

Height of Water Level above Rim of Test Basin (inches)

At beginning of interval: 4 H₁
 At end of interval: 4 H₂

3. **Test Data**

	<u>Time Begin,</u> T ₁	<u>Time End,</u> T ₂	<u>Test Length</u> (min)	<u>Δ Height</u> (in)
a.	0:00	02:00:00	120	0.000
b.	0:00	02:00:00	120	0.000
c.	0:00	02:00:00	120	0.000
d.	0:00	02:00:00	120	0.000
		av =	120	0

4. **Permeability Calculation** K (in/hr) = 60 min/hr * r²/R² * L (in)/T (min) * ln (H₁/H₂)
 K = 0.00 in/hr = Soil Permeability Class K0

5. Any **Defects** in Sample: No

6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water pollution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.

Signature of Professional Engineer _____

License #

TUBE PERMEAMETER TEST DATA

Project Name:	Hightstown Redevelopment	Project Number:	16001094A
Block:	21	Municipality:	Hightstown
Lot:	21	County:	Mercer
Test Number:	S-1	Date Collected:	6/15/2017
Material Tested:	TP-3	Replicate:	B
Depth of Sample:	68"	Sample Type:	Undisturbed

1. **Sample Dimensions** Radius = 2.54 cm
 Length = 4.750 in

2. **Measurements** Tube Weight = 368.01 g
 Total Weight = 830.87 g
 tube #: AV-1 Soil Weight = 462.86 g

Volume = Length * 2.54 cm/inch * π * Radius²
 Volume = 244.41

Bulk Density = Soil Weight / Volume
 Bulk Density = 1.89

Height of Water Level above Rim of Test Basin (inches)

At beginning of interval: 4.125 H₁
 At end of interval: 4.125 H₂

3. **Test Data**

	<u>Time Begin,</u> T ₁	<u>Time End,</u> T ₂	<u>Test Length</u> (min)	<u>Δ Height</u> (in)
a.	0:00	02:00:00	120	0.000
b.	0:00	02:00:00	120	0.000
c.	0:00	02:00:00	120	0.000
d.	0:00	02:00:00	120	0.000
		av =	120	0

4. **Permeability Calculation** K (in/hr) = 60 min/hr * r²/R² * L (in)/T (min) * ln (H₁/H₂)
 K = 0.00 in/hr = Soil Permeability Class K0

5. Any **Defects** in Sample: No

6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water pollution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.

Signature of Professional Engineer _____ License #

TUBE PERMEAMETER TEST DATA

Project Name:	Hightstown Redevelopment	Project Number:	16001094A
Block:	21	Municipality:	Hightstown
Lot:	21	County:	Mercer
Test Number:	S-1	Date Collected:	6/15/2017
Material Tested:	TP-4	Replicate:	A
Depth of Sample:	39"	Sample Type:	Undisturbed

1. **Sample Dimensions** Radius = 2.54 cm
 Length = 4.625 in

2. **Measurements** Tube Weight = 356.47 g
 Total Weight = 818.05 g
 tube #: Bm-43 Soil Weight = 461.58 g

Volume = Length * 2.54 cm/inch * π * Radius²
 Volume = 237.98

Bulk Density = Soil Weight / Volume
 Bulk Density = 1.94

Height of Water Level above Rim of Test Basin (inches)

At beginning of interval: 4 H₁
 At end of interval: 2.875 H₂

3. **Test Data**

	<u>Time Begin,</u> T ₁	<u>Time End,</u> T ₂	<u>Test Length</u> (min)	<u>Δ Height</u> (in)
a.	0:00	00:16:15	16.25	1.125
b.	0:00	00:14:30	14.5	1.125
c.	0:00	00:16:45	16.75	1.125
d.	0:00	00:17:30	17.5	1.125
		av =	16.25	1.125

4. **Permeability Calculation** K (in/hr) = 60 min/hr * r²/R² * L (in)/T (min) * ln (H₁/H₂)
 K = 5.64 in/hr = Soil Permeability Class K3

5. Any **Defects** in Sample: No

6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water pollution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.

Signature of Professional Engineer _____

License #

TUBE PERMEAMETER TEST DATA

Project Name:	Hightstown Redevelopment	Project Number:	16001094A
Block:	21	Municipality:	Hightstown
Lot:	21	County:	Mercer
Test Number:	S-2	Date Collected:	6/15/2017
Material Tested:	TP-4	Replicate:	B
Depth of Sample:	96"	Sample Type:	Undisturbed

1. **Sample Dimensions** Radius = 2.54 cm
 Length = 4.125 in

2. **Measurements** Tube Weight = 358.91 g
 Total Weight = 748.99 g
 tube #: Bm551 Soil Weight = 390.08 g

Volume = Length * 2.54 cm/inch * π * Radius²
 Volume = 212.25

Bulk Density = Soil Weight / Volume
 Bulk Density = 1.84

Height of Water Level above Rim of Test Basin (inches)

At beginning of interval: 3.875 H₁
 At end of interval: 3.875 H₂

3. **Test Data**

	<u>Time Begin,</u> T ₁	<u>Time End,</u> T ₂	<u>Test Length</u> (min)	<u>Δ Height</u> (in)
a.	0:00	02:00:00	120	0.000
b.	0:00	02:00:00	120	0.000
c.	0:00	02:00:00	120	0.000
d.	0:00	02:00:10	120	0.000
		av =	120	0

4. **Permeability Calculation** K (in/hr) = 60 min/hr * r²/R² * L (in)/T (min) * ln (H₁/H₂)
 K = 0.00 in/hr = Soil Permeability Class K0

5. Any **Defects** in Sample: No

6. I hereby certify that the information on Form 3b of this application is true and accurate. I am aware that falsification of data is a violation of the water pollution Control Act (NJSA 59:10A-1 et seq.) and is subject to penalties as prescribed in NJAC 7:14-8.

Signature of Professional Engineer _____

License #